

GENERAL STRUCTURAL NOTES

1.

BUILDING CODES USED FOR DESIGN:

a.

MINNESOTA RESIDENTIAL BUILDING CODE, CURRENT EDITION. (IBC 2018)
2.

DESIGN CRITERIA:

a.

GENERAL:

b.

BUILDING / STRUCTURE RISK CATEGORY:

II

WIND LOAD CRITERIA:

BASIC WIND SPEED (ULTIMATE)

V = 109 MPH

WIND LOAD EXPOSURE

C

WIND TOPOGRAPHIC FACTOR

Kzt = 1.0

INTERNAL PRESSURE COEFFICIENT

Gcpi = 0.18 (ENCLOSED)

c.

SNOW LOAD CRITERIA:

GROUND SNOW LOAD

Pg = 50 PSF

SNOW IMPORTANCE FACTOR

Is = 1.0

SNOW EXPOSURE FACTOR

Ce = 1.0

SLOPED ROOF / FLAT ROOF FACTOR

Cs = 1.0

ROOF THERMAL FACTOR

Ct = 1.1

SEE PLANS FOR SNOW DRIFT DIAGRAM

Ps = Pg(0.7)(Is)(Ce)(Cs)(Ct) = 38.5 PSF

d.

MISC. LIVE LOADS:

PUBLIC AREAS, AND CORRIDORS

100 PSF

OFFICE AREAS

15' + 50 PSF

*PARTITION LOAD

**OR EQUIPMENT WEIGHT IF HEAVIER
3.

DESIGN STRESSES:

a.

CONCRETE:

MEMBER TYPE/ LOCATION	STRENGTH @ 28 DAYS	EXPOSURE CLASSES	MAX W/C RATIO	MAXIMUM AGGREGATE	AIR CONTENT (SEE NOTE1)
FOOTING	4500 PSI	F2, S1, W2, C1	0.45	3/4"	6.0%
PIERS	5000 PSI	F3, S1, W2, C2	0.40	3/4"	6.0%
FOUNDATION WALLS	5000 PSI	F3, S1, W2, C2	0.40	3/4"	6.0%
SLAB	5000 PSI	F3, S1, W2, C2	0.40	1 1/2"	5.5%

NOTE:

FOR fc EXCEEDING 5000 PSI REDUCE OF AIR CONTENT BY 1.0% IS PERMITTED.

b.

REINFORCEMENT (DEFORMED BARS)

Fy = 60,000 PSI ASTM A615

c.

WELDED WIRE FABRIC

ASTM A185

4.

CONCRETE COVERAGE FOR REINFORCEMENT:

a.

FOOTINGS

3" FROM BOTTOM

b.

FOUNDATION WALLS

2" EXT. FACE, 1" INT. FACE

c.

PIER SIDES

2"

5.

FOUNDATIONS:

a.

ALL FOOTINGS SHALL BEAR ON NATURAL UNDISTURBED SOIL OR ON COMPACTED GRANULAR FILL. ALL FOOTINGS ARE DESIGNED USING AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF.

b.

GRANULAR FILL SHALL BE COMPACTED TO 98% STANDARD DENSITY (ASTM: D698).

c.

IF SOIL AT BOTTOM OF FOOTINGS AS DETAILED IS OF QUESTIONABLE BEARING VALUE, THE ARCHITECTS' OFFICE SHALL BE NOTIFIED AT ONCE.

d.

IT WILL BE THE RESPONSIBILITY OF OTHERS TO VERIFY THIS BEARING CAPACITY, TO ENSURE THAT DAMAGING DIFFERENTIAL SETTLEMENT WILL NOT OCCUR.

e.

ALL EXTERIOR WALL FOOTINGS ADJACENT TO HEATED STRUCTURES SHALL HAVE A MINIMUM SOIL COVER OF 3'-6" AND FOOTINGS AT UNHEATED STRUCTURES SHALL HAVE A MINIMUM SOIL COVER OF 5'-0". SOIL COVER IS MEASURED FROM BOTTOM OF FOOTING UNLESS OTHERWISE NOTED.

f.

PROVIDE A 6" SAND CUSHION AND POLY VAPOR BARRIER BENEATH ALL SLABS ON GRADE. COMPACT SAND WITH MECHANICAL EQUIPMENT TO +0" TO -3/4" OF CORRECT ELEVATIONS. THE VAPOR BARRIER SHALL BE PLACED DIRECTLY BENEATH THE SLAB. THE SLAB SHALL BE MOIST CURED TO PREVENT CURLING.

g.

BASE FILL (SAND CUSHION) FOR SLABS ON GRADE SHALL BE REASONABLY WELL GRADED SAND (SW OR SP) CLEAN AND FREE OF ORGANIC MATERIAL WITH NOT MORE THAN 5%, BY WEIGHT, PASSING A NO. 200 SIEVE AND LESS THAN 40%, BY WEIGHT, PASSING THE #40 SIEVE. COARSE AGGREGATE SHALL NOT EXCEED 3/4".

6.

BACKFILLING:

a.

NO BACKFILLING AND COMPACTING OF EARTH SHALL BE PERMITTED AGAINST FOUNDATION WALLS UNTIL SUPPORTING FLOOR SYSTEMS HAVE BEEN PLACED AND HAVE REACHED 75% OF THEIR DESIGN STRENGTH OR UNLESS ADEQUATE BRACING SUBMITTED FOR REVIEW IS PROVIDED.

b.

BOTH SIDES OF FOUNDATION WALLS SHALL BE BACKFILLED SIMULTANEOUSLY SO AS TO PREVENT OVERTURNING OR LATERAL MOVEMENT OF WALLS.

7.

REINFORCING STEEL:

a.

THE REINFORCING STEEL CONTRACTOR SHALL FABRICATE ALL REINFORCEMENT AND FURNISH ALL ACCESSORIES, CHAIRS, SPACER BARS AND SUPPORTS NECESSARY TO SECURE THE REINFORCEMENT UNLESS SHOWN OTHERWISE ON THE PLANS AND / OR DETAILS.

b.

CONCRETE REINFORCEMENT SHALL BE PLACED ACCORDING TO THE CRSI "RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS".

c.

COMPRESSION AND TENSION LAP SPLICES FOR CAST-IN-PLACE CONCRETE SHALL BE 38 BAR DIAMETER MINIMUM UNLESS OTHERWISE NOTED.

d.

HORIZONTAL REINFORCING STEEL IN FOOTINGS AND CONCRETE WALLS SHALL BE CONTINUOUS AROUND CORNERS.

e.

ALL LAPS IN WWF SHOULD BE ONE MESH PLUS TWO INCHES AT SPLICES.

f.

REINFORCING BARS MAY NOT BE WELDED WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER. ONLY ASTM A706 REINFORCEMENT MAY BE WELDED.

8.

CONCRETE:

a.

CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301.

b.

COMPLY WITH ACI 304 FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE.

c.

COMPLY WITH ACI 305 FOR HOT WEATHER CONCRETING.

d.

COMPLY WITH ACI 306 FOR COLD WEATHER CONCRETING.

e.

UNLESS SPECIFIED OTHERWISE, CONCRETE MUST REACH THE FOLLOWING PERCENTAGES OF ITS 28 DAY COMPRESSIVE STRENGTH (fc) BEFORE FORMS MAY BE REMOVED:

i.

WALLS, PIERS, AND FOOTINGS

40 PERCENT

9.

CONSTRUCTION AND CONTROL JOINTS IN CONCRETE:

a.

CONSTRUCTION JOINTS SHALL BE MADE AS DETAILED ON THE DRAWINGS.

b.

MAXIMUM SPACING FOR CONTROL JOINTS IN SLABS ON GRADE SHALL BE 15'-0".

c.

A 15'-0" MAXIMUM SPACING OF CONTROL JOINTS MAY NOT ENSURE COMPLETE CONTROL OF SHRINKAGE CRACKS. CLOSER SPACING MAY BE USED BY REQUEST OF OWNER IF MORE COMPLETE SHRINKAGE CRACK CONTROL IS DESIRED. CONTRACTOR TO VERIFY WITH OWNER.

d.

CONSTRUCTION JOINTS IN CONCRETE FOUNDATION WALLS SHALL BE LOCATED SO NO SINGLE POUR IS LONGER THAN 40 FEET.
10.

DIMENSION LUMBER

a.

DIMENSION LUMBER TO BE NORTHERN SPF NO. 2 (OR BETTER) FOR JOISTS & BEAMS AND NORTHERN SPF STUD GRADE (OR BETTER) FOR STUDS AND PLATES.

b.

ALL MEMBER SIZES GIVEN ON PLAN ARE NOMINAL DIMENSIONS.

c.

WOOD LINTELS SHALL HAVE A FULL 3" LENGTH OF BEARING AT EACH END UNLESS OTHERWISE NOTED.

d.

ALL NAILING SHALL CONFORM TO IBC TABLE 2304.10.1 "FASTENING SCHEDULE" UNLESS OTHERWISE NOTED ON PLANS.

e.

SPACING OF BRIDGING FOR FLOOR AND ROOF JOISTS SHALL NOT EXCEED 8' OR 6 TIMES THE NOMINAL JOIST DEPTH (WHICHEVER IS GREATER).

f.

DOUBLE ALL JOISTS UNDER PARALLEL PARTITIONS.

g.

ALL WOOD CONNECTORS SHALL BE BY "USP LUMBER CONNECTORS" OR "SIMPSON STRONG-TIE". ALL JOISTS AND BEAMS NOT BEARING ON A SUPPORTING MEMBER SHALL BE FRAMED WITH AN APPROPRIATE WOOD CONNECTOR.

h.

WOOD JOISTS SHALL BEAR ON THE FULL WIDTH OF SUPPORTING MEMBERS (STUD WALLS, BEAMS, ETC.), UNLESS NOTED OTHERWISE.

i.

PROVIDE SOLID BLOCKING BELOW ALL JAMB / TRIMMER / CRIPPLE STUDS (TYPICAL AT ALL FLOORS).

j.

ALL FOUNDATION PLATES, SILLS AND SLEEPERS ON CONCRETE SLAB, WHICH IS IN DIRECT CONTACT WITH EARTH, AND SILLS WHICH REST ON CONCRETE OR MASONRY FOUNDATION WALLS, SHALL BE TREATED WOOD.

k.

FOR ALL WOOD TREATED WITH PRESERVATIVES OTHER THAN CCA, CONNECTORS AND FASTENERS MUST BE COATED WITH ONE OF THE FOLLOWING:

i.

BATCH / POST HOT DIPPED GALVANIZED PER ASTM A123 FOR CONNECTORS AND ASTM 153 FOR FASTENERS.

ii.

MECHANICALLY GALVANIZED PER ASTM 695, CLASS S5 OR GREATER.

iii.

TRIPLE ZINC G185 HDG PER ASTM A653 OR EQUAL.
11.

BRACED WALL PANELS (SHEAR WALLS) IN WOOD CONSTRUCTION:

a.

UNLESS NOTED OTHERWISE ON THE PLANS, ALL INTERIOR PARTITION WALLS WITH GYPSUM BOARD SHEATHING ARE DESIGNED TO PROVIDE LATERAL STABILITY TO THE STRUCTURE. THIS INCLUDES WALLS SHOWN ON THE ARCHITECTURAL PLANS AND NOT NECESSARILY SHOWN ON THE STRUCTURAL PLANS.
12.

ROOF TRUSSES:

a.

TRUSSES SHALL BE DESIGNED TO MEET ALL LOADING AND SPANS AS INDICATED ON THE PLANS.

b.

TRUSSES SHALL BE DESIGNED AND CERTIFIED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF THE PROJECT.

c.

ALL PERMANENT BRACING FOR INDIVIDUAL TRUSS COMPRESSION ELEMENTS SHALL BE PROVIDED AS INDICATED ON THE TRUSS SHOP DRAWINGS. THE DESIGN OF THIS BRACING IS THE RESPONSIBILITY OF THE TRUSS SUPPLIER.

d.

THE CONTRACTOR SHALL INSTALL ALL NECESSARY TEMPORARY BRACING AS REQUIRED BY BCSI 1-03 (BY WTCA AND TPI) AND BE FULLY RESPONSIBLE FOR THE STABILITY OF THE TRUSSES DURING ERECTION.

e.

CONNECTOR PLATES SHALL BE MADE OF GRADE "A" GALVANIZED STEEL, MINIMUM 20 GAUGE PER LATEST TPI SPECIFICATIONS.

f.

ALL CONNECTION HARDWARE SHALL BE DESIGNED AND FURNISHED BY THE TRUSS SUPPLIER UNLESS NOTED OTHERWISE ON THE PLANS.

g.

THE STRUCTURE IS DESIGNED ACCORDING TO THE TRUSS LAYOUT INDICATED ON THE PLANS. THE TRUSS SUPPLIER SHALL NOT DEVIATE FROM THIS LAYOUT WITHOUT PERMISSION FROM THE ENGINEER OF RECORD.

h.

ROOF TRUSSES SHALL BE DESIGNED FOR UNBALANCED SNOW LOADS IN ACCORDANCE WITH ASCE 7, SECTION 7.6.

i.

TRUSSES SHALL BE DESIGNED FOR A TOP CHORD DEAD LOAD OF 10 PSF AND A BOTTOM CHORD DEAD LOAD OF 10 PSF UNLESS OTHERWISE ON THE PLANS.
13.

BRICK TIES:

a.

THERE SHALL BE A MINIMUM OF ONE BRICK TIE FOR EVERY 1.75 SQ. FT. OF WALL AREA. THESE SHALL BE SPACED AT A MAXIMUM OF 24 INCHES ON CENTER. TIES SHALL BE OF MINIMUM 3/16" GA. CORROSION RESISTANT WIRE AND SHALL BE OF AN ADJUSTABLE TYPE SUCH AS DUR-O-WALL ADJUSTABLE DA207 OR EQUAL. CORRUGATED GALVANIZED SHEET TIES ARE NOT ACCEPTABLE. ALL TIES MUST BE ATTACHED THROUGH THE SHEATHING TO THE STUDS PER MANUFACTURERS RECOMMENDATIONS.
14.

CONSTRUCTION PROCEDURE:

a.

THE STRUCTURE SHALL BE ADEQUATELY BRACED AND SHORED DURING ERECTION AGAINST WIND AND ERECTION LOADS. STRUCTURAL MEMBERS ARE DESIGNED FOR 'IN PLACE' LOADS. COMPLY WITH ALL APPLICABLE CITY, COUNTY, STATE AND FEDERAL LAWS, INCLUDING THE OCCUPATIONAL SAFETY AND HEALTH ACT (OSHA) AND REGULATIONS ADOPTED PURSUANT THERETO.

b.

THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS OTHERWISE NOTED, THEY DO NOT INDICATE THE MEANS OR METHOD OF CONSTRUCTION. PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKMEN OR OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT ARE NOT LIMITED TO, BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR THE BUILDING, SHORING FOR EARTH BANKS, FORMS, SCAFFOLDING, PLANKING, SAFETY NETS, SUPPORT AND BRACING FOR CRANES AND GIN POLES, ETC.

d.

ENGAGE PROPERLY QUALIFIED PERSONS TO DETERMINE WHERE AND HOW TEMPORARY PRECAUTIONARY MEASURES SHALL BE USED AND INSPECT SAME IN THE FIELD. OBSERVATION VISITS TO THE SITE BY ENGINEER'S FIELD REPRESENTATIVE SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS.

e.

SUPERVISE AND DIRECT THE WORK SO AS TO MAINTAIN SOLE RESPONSIBILITY FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES. AS A PART OF THIS RESPONSIBILITY, RETAIN THE SERVICES OF A LICENSED STRUCTURAL ENGINEER TO DESIGN AND SUPERVISE ANY SCAFFOLDING FOR WORKMEN, AND ALL SHORING OF FORMS AND ELEMENTS OF THE CONSTRUCTION.
15.

MISCELLANEOUS:

a.

PLACEMENT OF ANCHOR BOLT, PIPE SLEEVES, PADS AND OPENINGS FOR EQUIPMENT SHALL BE COORDINATED BETWEEN THE GENERAL CONTRACTOR AND THE OTHER SUBCONTRACTORS.

b.

ALL CORE DRILLING SHALL BE DONE UNDER THE SUPERVISION OF THE GENERAL CONTRACTOR. NO REINFORCING SHALL BE CUT. VERIFY LOCATION OF REINFORCING BEFORE CORE DRILLING. THERE SHALL NOT BE ANY CORE DRILLING THROUGH BEAMS OR COLUMNS. MAXIMUM CORE HOLE THROUGH SLABS SHALL BE PIPE DIAMETER PLUS 1".
16.

COORDINATION WITH ARCHITECTURAL DRAWINGS:

a.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. WHERE DISCREPANCIES OCCUR, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO CONSTRUCTION.
17.

SHOP DRAWINGS:

a.

SHOP DRAWINGS, UNLESS OTHERWISE NOTED, SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION.

b.

SHOP DRAWINGS SHALL BE PREPARED UNDER THE SUPERVISION OF A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF THE PROJECT, AND INCLUDE COMPLETE DETAILS, SCHEDULES, PROCEDURES AND DIAGRAMS FOR FABRICATION AND ASSEMBLY OF STRUCTURAL MEMBERS.

c.

FABRICATORS SHALL DRAW THEIR OWN ERECTION PLANS. COPYING THE STRUCTURAL PLANS AND USING THEM AS ERECTION DRAWINGS IS NOT ACCEPTABLE.

d.

PRIOR TO SUBMITTAL, THE CONTRACTOR SHALL REVIEW THE SHOP DRAWINGS AND MAKE ANY CORRECTIONS REQUIRED. THE CONTRACTOR SHALL STAMP AND SIGN THE DRAWINGS AS EVIDENCE THAT HE HAS REVIEWED THEM.

e.

SHOP DRAWINGS SHALL BE FURNISHED FOR ALL STRUCTURAL COMPONENTS.

f.

TURN AROUND TIME FOR SHOP DRAWINGS SHALL BE TWO WEEKS FROM DATE RECEIVED IN THE ENGINEER'S OFFICE.
18.

SPECIAL INSPECTIONS:

SPECIAL INSPECTIONS SHALL BE PROVIDED IN ACCORDANCE WITH IBC CHAPTER 17. THE SPECIAL INSPECTOR SHALL BE EMPLOYED BY THE OWNER, SHALL BE THOROUGHLY KNOWLEDGEABLE OF IBC SPECIAL INSPECTION REQUIREMENTS AND SHALL DEMONSTRATE COMPETENCE TO THE SATISFACTION OF THE BUILDING OFFICIAL (IBC 1704). THE CONTRACTOR SHALL CONTACT THE SPECIAL INSPECTOR DURING APPROPRIATE PHASES OF CONSTRUCTION SO THAT INSPECTIONS CAN BE MADE IN A TIMELY MANNER. THE SPECIAL INSPECTOR SHALL SUBMIT WRITTEN INSPECTION REPORTS TO THE ENGINEER OF RECORD'S OFFICE, WITHIN 3 WORKING DAYS OF EACH INSPECTION. ANY PROBLEMS SHOULD BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR. THE FOLLOWING ITEMS WILL REQUIRE SPECIAL INSPECTION:

a.

CONCRETE:

i.

REINFORCEMENT: REINFORCING STEEL SHALL BE INSPECTED ON A PERIODIC BASIS. WELDING OF REINFORCEMENT SHALL BE CONTINUOUSLY INSPECTED. ONLY ASTM A706 REINFORCEMENT MAY BE WELDED.

ii.

SAMPLING AND TESTING: CONTINUOUS INSPECTIONS SHALL BE PROVIDED DURING SLUMP TESTS, AIR CONTENT TESTS AND WHEN DETERMINING THE TEMPERATURE OF FRESH CONCRETE AT THE TIME OF MAKING SPECIMENS FOR STRENGTH TESTS.

iii.

CONCRETE PLACEMENT: CONTINUOUS INSPECTION REQUIRED.

iv.

COLD AND HOT WEATHER CONCRETING: PERIODIC INSPECTION OF COMPLIANCE IS REQUIRED, IF APPLICABLE.

19.

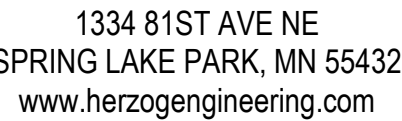
TESTING REQUIREMENTS:


a.

CONCRETE:

i.

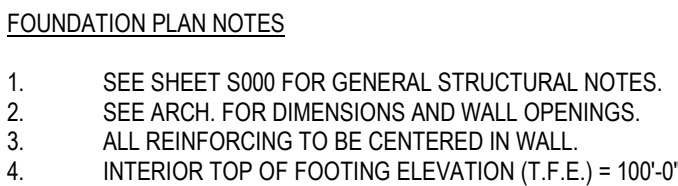
SAMPLE FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN NOT LESS THAN ONCE A DAY, NOR LESS THAN ONCE FOR EACH 150 CUBIC YARDS OF CONCRETE, NOR LESS THAN ONCE FOR EACH 5000 SQUARE FEET OF SURFACE AREA FOR SLABS OR WALLS. A MINIMUM OF FIVE STRENGTH TESTS SHOULD BE MADE FOR A GIVEN PROJECT.
-
- 1334 81ST AVE NE
SPRING LAKE PARK, MN 55432
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- PRINCETON DENTAL CENTER
PRINCETON, MN
- | No. | Description | Date |
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- I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA
- PRINT NAME: Jeremy J. Clarke
- SIGNATURE: *Jeremy J. Clarke*
- DATE: 04/29/2025 REGISTRATION # 42422
- GENERAL
STRUCTURAL
NOTES
- | | |
|----------------|-----------|
| Project Number | 251145 |
| Date | 4/29/2025 |
| Drawn By | MT |
| Checked By | JC/MW |
- S000
- Scale
- SHEET INDEX
- | SHEET NUMBER | SHEET NAME |
|--------------|--------------------------|
| S000 | GENERAL STRUCTURAL NOTES |
| S100 | FOUNDATION PLAN |
| S200 | ROOF FRAMING PLAN |
| S300 | FOUNDATION DETAILS |
| S400 | FRAMING DETAILS |

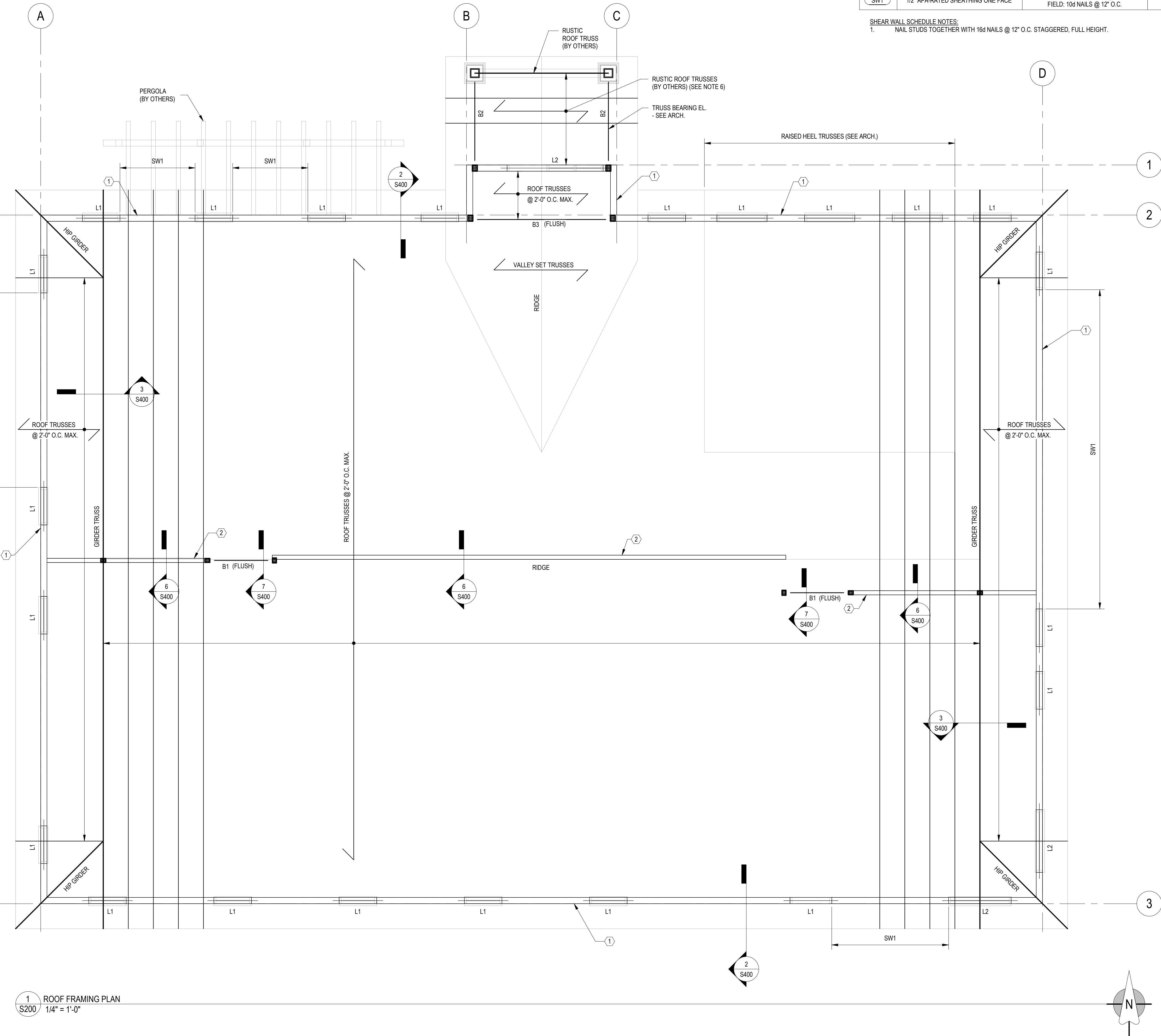
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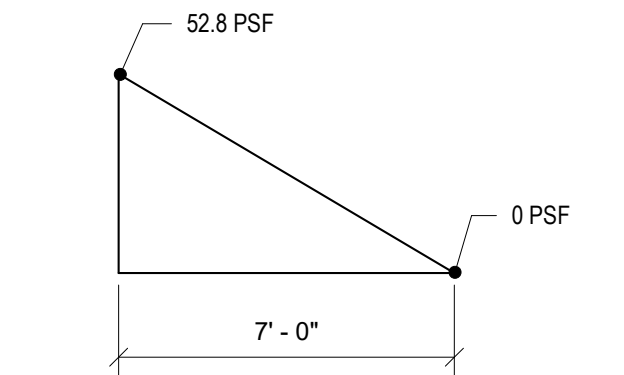
Scale $1/4" = 1'-0"$





SHEAR WALL SCHEDULE					
MARK	SHEATHING	FASTENERS	SILL PLATE FASTENERS BETWEEN HOLD DOWNS	SHEAR WALL END POSTS	HOLD DOWN EACH END OF WALL
SW1	1/2" APA-RATED SHEATHING ONE FACE	PANEL EDGES: 10d NAILS @ 6" O.C. FIELD: 10d NAILS @ 12" O.C.	5/8" Ø ANCHOR BOLTS @ 24" O.C.	2 PLY - 2x6	SIMPSON HDU2 - SDS2.5 W/ 5/8" Ø HILTI HAS-E THREADED ROD W/ HIT-HY 200 V3 EPOXY, 5" EMBED

SHEAR WALL SCHEDULE NOTES:
1. NAIL STUDS TOGETHER WITH 16d NAILS @ 12" O.C. STAGGERED, FULL HEIGHT.



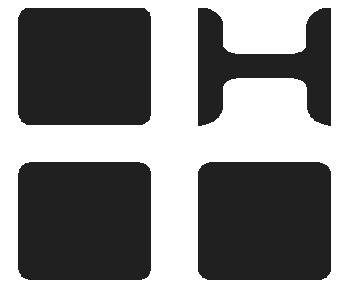
2 SNOW DRIFT LOAD DIAGRAM
S200 1/4" = 1'-0"

LINTEL SCHEDULE		
MARK	SIZE	JAMBS
L1	(3) 2x8	(2) JACK, (1) KING
L2	(3) 2x10	(2) JACK, (2) KING

BEAM SCHEDULE		
MARK	SIZE	COMMENTS
B1	(2) 2x10	<varies>
B2	4x6 TREATED	-
B3	(2) 1 3/4x9 1/2" LVL	-

- ROOF FRAMING PLAN KEY NOTES
- 2x6 STUDS @ 16" O.C.
 - 2x4 STUDS @ 16" O.C. BEARING WALL
 - SNOW DRIFT LOAD, SEE S200

- ROOF FRAMING PLAN NOTES
- SEE SHEET S300 FOR GENERAL STRUCTURAL NOTES.
 - SEE 1/S400 FOR TYPICAL SHEATHING FASTENING DETAIL.
 - TRUSS BEARING EL. (T.B.E.) = 110'-1 1/8".
 - ROOF SHEATHING SHALL BE MINIMUM 1/2" W/ 32/16 SPAN RATING.
 - SEE 2/S105 FOR FASTENING DETAIL.
 - SEE ARCH. FOR DIMENSIONS FOR WALLS, WALL OPENINGS, AND MORE INFORMATION.
 - WOOD ROOF DECKING TO RUN PERPENDICULAR TO TRUSSES SEE ARCH. FOR ADDITIONAL INFO.



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PRINCETON, MN

No.	Description	Date
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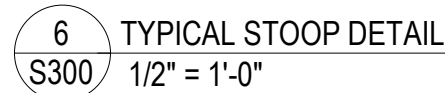
PRINT NAME: Jeremy J. Clarke
SIGNATURE: *Jeremy J. Clarke*
DATE: 04/29/2025 REGISTRATION # 42422

ROOF FRAMING
PLAN

Project Number	251145
Date	4/29/2025
Drawn By	MT
Checked By	JC/MW

S200

Scale As indicated





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SIGNATURE: Jerry A. Clark

FRAMING DETAILS

Checked By JC/MW

Scale	As indicated
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